# ST AMBROSE ANGLICAN CHURCH, GILGANDRA, NSW

# FOUNDATION UNDERPINING

#### GENERAL

G1.	THE BUILDER SHALL BE RE	SPONSIBLE FOR MAINTAINING STABILITY OF THE STRUCTURE UNTIL
<u></u>		
62.		
	STABILITY OF THE STRUCT	URE FORMWORK CRANERASE TEMPORARY WORKING
	PLATEORMS FACADE RET	ENTION SYSTEMS AND GROUND IMPROVEMENT TO SUPPORT
	CONSTRUCTION PLANT	
	THE DESIGN OF ALL TEMPO	RARY WORKS WILL BE UNDERTAKEN BY A QUALIFIED TEMPORARY
	WORKS ENGINEER APPOIN	TED BY THE CONTRACTOR
G3.	STRUCTURAL DRAWINGS A	RE TO BE READ IN CONJUNCTION WITH ALL ARCHITECTURAL &
	OTHER CONSULTANTS DRA	WINGS & SPECIFICATIONS.
G4.	ALL MATERIALS AND WORK	MANSHIP SHALL BE IN ACCORDANCE WITH THE LATEST REVISIONS
	OF THE FOLLOWING CODE	S EXCEPT WHERE VARIED BY THE SPECIFICATION AND / OR
	DRAWINGS.	
	AS.1720	TIMBER STRUCTURES
	AS.2159	PILING CODE
	AS.3600	CONCRETE STRUCTURES
	AS.3610	FORMWORK FOR CONCRETE
	AS.3700	MASONRY STRUCTURES
	AS.4100	STEEL STRUCTURES
	AS.2269	STRUCTURAL PLYWOOD
	NATIONAL CONSTRU	ICTION CODE OF AUSTRALIA
G5.	DIMENSIONS NOT TO BE SO	
$\gamma \gamma \gamma$	SETYOUT DIMENSIONS ARE	TO BE VERIFIED WITH ARCHITECT. $\land \land \land$

G6. WHERE BLIGH TANNER ARE REQUIRED TO PROVIDE CONSTRUCTION CERTIFICATION. THE CONTRACTOR IS TO ADVISE THE STRUCTURAL ENGINEER OF COMMENCEMENT OF CONSTRUCTION AND PROVIDE MINIMUM 24HRS NOTICE SO THAT INSPECTIONS OF STRUCTURAL ELEMENTS CAN BE UNDERTAKEN. FAILURE TO NOTIFY THE ENGINEER OF WORK AND FACILITATE AN INSPECTION WILL RESULT IN WORKS BEING EXCLUDED FROM CERTIFICATION

#### FOUNDATIONS

F1.	REFER TO GEOTECHN	IICAL REPORT BY:		Γ	1
	GEOTECHNICAL ENG	GINEER	GEOTECHNICAL REPORT	DATE	
	BARNSON		40088-GR01_D	JULY 2024	
	FOR GROUND CONDIT	IONS			1
F2.	FOUNDATION EXCAVA	TIONS TO BE MAI	NTAINED IN A FIRM	DRY CONDITION.	REMOVE ANY SOFT
F3.	EXCAVATION SHALL N THE EXISTING FOOTIN LINE OF INFLUENCE:	OT BE PERFORME IG.	Ed Below the Lin	E OF INFLUENCE E	XTENDING FROM
	IN ROCK		- 2 VERTICAL TO	) 1 HORIZONTAL	
	IN SILT OR CLA		- 1 VERTICAL TO	) 1 HORIZONTAL	
F1		HER MATERIAL		D Z HURIZUNTAL PORTING EXCAVAT	
F5	ALL FOOTINGS SHALL	BE COMPACTED I	N ACCORDANCE W		AL REPORT
	RECOMMENDATIONS	AND FOUNDED ON	NATURAL UNDIST	URBED MATERIAL.	
	FOOTING	MINIMUM ALLO	WABLE ACITY		
	CONCRETE PILES	364kPa INTO HARD CLA	Y		
F6.	BUILDER TO ENGAGE	CERTIFIED GEOTI	ECHNICAL ENGINE	ER TO CONFIRM M	INIMUM
F7.	STRUCTURAL FILL TO	BE NON REACTIV	E CBR15 PLACED I	N 200 THICK LOOS	E LAYERS &
	uuu				w

#### CONCRETE

C1. C2.	CONCRETE SPECIFICATION SLUMP MAXIMUM AGGREGAT CEMENT PROJECT CONTROL TESTING SPECIFICATION TEST REPOR CONCRETE STRENGTH & CL	TE G SHALL BE CA RTS TO BE SUE EAR COVER ( I	80mm 20mm TYPE 'A' PORT ARRIED OUT IN 3MITTED TO TH NCLUDING FIT	LAND ACCORDANCI HE PRINCIPAL MENTS ) TO BI	E WITH AS.3 FOR APPRO E AS FOLLO
	ELEMENT CONCRETE COVER				
		GRADE	BOTTOM	TOP	SIDES
	BORED PIERS FOOTINGS SLAB ON GROUND	N32 N32	50 50	50 50	65 50
	- INTERNAL - EXTERNAL	N32 N32	40 40	40 40	40 40
	WALLS	N32	-	40	40

ALL SLABS TO BE SPRAYED WITH ALIPHATIC ALCOHOL BARRIER AFTER INITIAL SCREEDING C3. AND BULLFLOATING. ALIPHATIC ALCOHOL TO BE RE-APPLIED AFTER EACH SUBSEQUENT FINISHING OPERATION.

CURE ALL CONCRETE SURFACES IN ACCORDANCE WITH AS.3600. CURING OF ALL CONCRETE C4. IS TO BE ACHIEVED BY KEEPING SURFACES CONTINUOUSLY WET FOR A PERIOD OF 3 DAYS AND PREVENTION OF LOSS OF MOISTURE FOR A TOTAL OF 7 DAYS FOLLOWED BY A GRADUAL DRYING OUT. USE OF AN APPROVED CURING AGENT. APPLY CURING COMPOUNDS WITHIN TWO HOURS OF THE FINISHING OPERATION.

ALL RE-ENTRANT CORNERS AND PENETRATIONS LARGER THAN 200 SQUARE ARE TO HAVE C5. TRIMMER BARS PLACED DIAGONALLY AT CORNERS. TRIMMER BARS SHALL BE 2 N12 x 1200 LONG AT 100 CENTRES. TOP & BTM FOR SUSPENDED SLABS. FOR SLABS ON GROUND PROVIDE TRIMMER BARS FOR EACH LAYER OF MESH.

ALL HOOKS AND BENDS TO BE IN ACCORDANCE WITH AS.3600. C6. UNLESS NOTED OTHERWISE ALL LAPS TO BE :

		STRUCTURAL ELEM	IENT	
SIZE	STAIRS, LANDINGS &	BEAMS, WALLS &	COLUMNS	BORED PIERS
0.22	SLABS ≤300 THICK	SLAB >300 THICK		FOOTINGS
N12	450	600	500	600
N16	700	900	650	900
N20	950	1200	850	1200
N24	1200	1600	1000	1600
N28	1500	2000	1200	2000
N32	1800	2400	1400	2400

MESH LAP TO BE 2 OUTER MOST CROSS BARS OF 1 SHEET LAPPED WITH 2 OUTER MOST BARS OF THE 2ND SHEET OF SIMILAR BAR SPACING

C7. USE PLASTIC OR PLASTIC TIPPED BAR CHAIRS AT 600 CENTRES FOR SL72 & 82 MESH AND AT 800 CENTRES FOR SL92 MESH & GREATER. C8. CONSTRUCTION JOINTS WHERE NOT SHOWN ON DRAWINGS SHALL BE LOCATED TO THE

APPROVAL OF THE ENGINEER AND PRINCIPAL. BASIC DRYING SHRINKAGE STRAIN MEASURED IN ACCORDANCE WITH AS.1012 - PART 13 C9.

SHALL NOT EXCEED 800 µm. C10. FOR FALLS, STEPS, CHAMFERS, DRIP GROOVES, REGLETS ETC. REFER TO ARCHITECTS

DETAILS. MAINTAIN COVER TO REINFORCEMENT AT THESE DETAILS. CONDUITS, PIPES, ETC SHALL ONLY BE LOCATED IN THE MIDDLE ONE THIRD OF ARCHITECTS C11.

SLAB DEPTH AND SPACED AT NOT LESS THAN 3 DIAMETERS.

C12. REINFORCEMENT SYMBOLS

'N' - DENOTES GRADE 500 BARS TO AS.4671. 'R' - DENOTES GRADE 250 HOT ROLLED PLAIN BARS TO AS.4671.

BAR GRADE AND TYPE CENTRES IN mm N 20 - 250 SL82 NOMINAL BAR SIZE IN mm ——— — BAR CENTRES x 100 mm 

C13. REINFORCEMENT LENGTHS TO BE DETERMINED WITH REFERENCE TO THE GEOMETRY AND SETOUT ON THE ARCHITECTS DRAWINGS.

C14. PROVIDE 50mm BEDDING SAND & DAMP PROOF MEMBRANE UNDER ALL GROUND SLABS. C15. ALL CONCRETE TO BE VIBRATED DURING PLACEMENT.

C16. ALL REINFORCEMENT TO BE SECURELY TIED PRIOR TO PLACEMENT OF CONCRETE.

C17. PROPPING AND FORMWORK TO BE IN ACCORDANCE WITH AS.3610. C18. NO CONCRETE IS TO BE POURED ON THE SITE WHEN TEMPERATURES EXCEED 35°C OR FALL

BELOW 5°C. C19. THE REINFORCEMENT STEEL SUPPLIER MUST BE CERTIFIED BY ARCS (AUSTRALIAN STANDARDS CERTIFICATION & VERIFICATION OF REINFORCING, PRESTRESSING AND STRUCTURAL STEELS) FOR THE SUPPLY OF REINFORCEMENT STEEL. CURRENT ARCS CERTIFICATION CERTIFICATES ARE TO BE ISSUED TO BLIGH TANNER PRIOR TO DELIVERY OF REINFORCING STEEL TO SITE. REFER http://steelcertification.com FOR CURRENT CERTIFICATE

HOLDERS. C20. WHERE CONCRETE SURFACES ARE EXPOSED AND EXPRESSED ARCHITECTURALLY NATURAL SAND IS TO BE USED IN THE CONCRETE MIX IN LIEU OF MANUFACTURED SAND.

C21. WHERE COLOURED CONCRETE IS SPECIFIED A TRIAL RUN AND TEST PANEL OF THE FINISHED CONCRETE IS TO BE COMPLETED FOR APPROVAL BY THE ARCHITECT AND ENGINEER. CORE SAMPLE OF THE TRIAL RUN ARE TO BE TAKEN AND CRUSHED TO CONFIRM SPECIFIED CONCRETE STRENGTH HAS BEEN ACHIEVED.

#### UNDERPINNING

U1. SUITABLE TEMPORARY WORKS ARE TO BE IN PLACE AND REVIEWED BY A QUALIFIED TEMPORARY WORKS ENGINEER PRIOR TO UNDERMINING THE EXISTING FOUNDATIONS. REFER TO INDICATIVE TEMPORARY PROPPING PLAN ON SHEET SSK10.

#### AS.3600 AND THE PPROVAL. OLLOWS

STRUCTURAL ENGINEER IS TO BE PROVIDED PRIOR TO REMOVING THE TEMPORARY PROPPING **U3. WHERE NO JACKING REQUIRED** 

U2. FOLLOWING COMPLETION OF THE UNDERPINNING WORKS, CONFIRMATION FROM THE

- (i) EXCAVATE AND CAST NEW FOOTINGS AS PER PLAN.
- (ii) REPEAT FOR NEXT SUCCESSIVE FOUNDATION SECTONS, ALLOWING 48 HOURS BETWEEN COMPLETION OF DRY PACKING & COMMENCEMENT OF THE EXCAVATION OF THE NEXT AREA
- U4. WHERE JACKING IS TO BE ATTEMPTED
- (i) EXCAVATE AND CAST NEW FOOTINGS TO SECTIONS AS PER PLAN

RE-INSTALL TEMPORARY PACKING SHIMS AND REMOVE JACKS.

- (ii) AFTER 24 HOURS, INSTALL TEMPORARY PACKING SHIMS BETWEEN NEW FOUNDATIONS AND UNDERSIDE OF EXISTING STRUCTURE.
- (iii) REPEAT STEPS (i) AND (ii) FOR NEXT SUCCESSIVE SECTION MARK NUMBER
- (iv) INSTALL JACKS ACROSS BUILDING FOOTPRINT OR WITHIN SELECTED AREAS. CAREFULLY AND PROGRESSIVELY INFLATE JACKS SO AS TO REVERSE ANY SETTLEMENT EFFECTS. DO NOT RAISE ANY JACK MORE THAN 5mm VERTICALLY WITHOUT THOROUGHLY CHECKING FOR
  - ADVERSE DISTRESS EFFECTS TO THE STRUCTURE. (v) ONCE JACKING OPERATIONS ARE COMPLETE TO THE SATISFACTION OF THE ENGINEER,
  - (vi) INSTALL NON-SHRINK DRY PACK AS REQUIRED BETWEEN NEW FOUNDATIONS AND UNDERSIDE OF EXISTING STRUCTURE
  - U5. CONTRACTOR SHALL EXCAVATE UNDER EXISTING STRIP FOOTINGS IN AN ORDERED SEQUENCE. EXISTING GROUND ON UNDERSIDE EITHER SIDE OF THE NEW FOOTINGS TO REMAIN IN TACT TO PROVIDE FULL SUPPORT TO EXISTING FOOTING UNTIL UNDERPINNING SECTION HAVE CURED FOR A MINIMUM OF 5 DAYS
  - U6. UNDERPINNING CONCRETE TO BE PLACED TIGHT TO UNDERSIDE OF EXISTING FOOTING. ENSURE THERE ARE NO VOIDS PRESENT. USE LETTER BOX SHUTTER OR DRY PACK HIGH STRENGTH 50MPa GROUT
  - U7. ROCK BEARERS OR HAMMERS ARE NOT PERMITTED. VIBRATION SHALL BE LIMITED TO A MAXIMUM 5mm/s PARTICLE VELOCITY. SUITABLE MONITORING SHALL BE INSTALLED.

#### SCREW PILES

- THS
- SP3. PILES TO COMPLY WITH THE FOLLOWING:
- 50mm MAXIMUM DESIGN ECCENTRICITY
- SP5. PILES TO HAVE A DESIGN LIFE OF 50 YEARS.
- SP6. ALL PILES TO BE CONCRETE CORE FILLED WITH A MINIMUM 32MPa CONCRETE WITH 6mm AGGREGATE. PILES TO BE CUT OFF 150mm ABOVE UNDERSIDE OF PILE CAP AND PILES TO BE CLOSED ENDED TYP.
- SP7. PROVIDE "AS BUILT" DRAWINGS WITH SURVEYED FINAL LOCATIONS

SP1.	ALL PILES TO BE DESIGNED, MANUFACTURED AND INSTALLED BY THE PILING CONTRACTOR
	ALL PILES TO BE CERTIFIED (SUPPLY FORMS 15 & 16) FOR DESIGN AND INSTALLATION BY A
	PRACTICING PROFESSIONAL ENGINEER.
	NOTE THAT INSTALLATION INCLUDES TRIMMING AND REMOVING OF EXCESS LENGTI
	AND INSTALLING DOWEL HEAD PINS).
SP2	EFFECTIVE LENGTH TO BE DETERMINED BY THE DESIGNER (MINIMUM 6m)

- 50mm MAXIMUM INSTALLATION TOLERANCES
- SP4. ALLOW FOR A MINIMUM CORROSION RATE OF 0.02mm / YEAR.

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ARCHITECT				
	JDA	A Co.		
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PROJECT				
ST AMBROSE ANGLICAN CHURCH 94 WAMBOIN STREET, GILGANDRA, NSW				
DRAWING TITLE				
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PINTING REQUIREMENTS

#### **GENERAL NOTES:**

1. TEMPORARY PROPPING OF EXISTING MASONRY ARCHES DURING UNDERPINNING BY TEMPORARY WORKS ENGINEER ENGAGED BY THE CLIENT

NEW TIMBER BEARERS TO INCORPORATE JACKING SYSTEM TO ALLOW FOR FUTURE ADJUSTABILITY OF BEARERS, RECOGNISING THAT FUTURE BUILDING MOVEMENT OF STRUCTURE FOUNDED AT HIGH LEVEL IS LIKELY

### SCREW PILE LEGEND:

MARK MAX AXIAL LOAD	MAX MOMENT DEMAND	COMMENT
SP1 310kN	5kNm	MIN 168 x 6.0mm CHS G350
SP2 450kN	10kNm	MIN 168 x 6.0mm CHS G350
SP3 150kN	45kNm	MIN 219 x 6.0mm CHS G350

\* PILES, INCLUDING CONNECTION TO PILE CAPS, TO BE DESIGNED BY CONTRACTOR WITH REFERENCE TO THE GEOTECHNICAL REPORT TO ACHIEVE THE GIVEN PILE LOAD. PROPOSED FOUNDATION CONNECTION TO BE PROVIDED TO BLIGH TANNER FOR REVIEW PRIOR TO CONSTRUCTION

\* SCREW PILE LOADS PROVIDED ARE TO SUPPORT GRAVITY LOADS FROM STRUCTURE

### GROUND BEAMS LEGEND:

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( CONTRACTOR CONTRACTO

	DIMENS	IONS		RE	INFORCEME	:NT
MARK	WIDTH	DEPTH	A	B	C	LIGS
GB1	600	600	4N24	4N24	-	2 LEGS N12-200 ctrs
GB2	800	800	5N24	4N20	2N20	2 LEGS N12-300 ctrs

### BORED PIER:

ONLY

MARK	<u>DIAMETER</u>	<u>DEPTH</u>
BP-A	450	1500
BP-B	450	6800



# FOUNDATION PLAN SCALE 1:100

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#### **UNDERPINNING WITH SCREW PILES - 2 WALLS** Scale 1:20



#### **TYPICAL GROUND BEAM DETAILS** Scale 1:20

NOTE: SEQUENCE FOR UNDERMINING AND CONSTRUCTING THE NEW FOUNDATIONS TO BE DEVELOPED IN COOPERATION WITH THE CONTRACTOR



#### **GROUND BEAMS LEGEND:**

DIMENSIONS			REINFORCEMENT			
MARK GB1 GB2	WIDTH 600 800	DEPTH 600 800	A 4N24 5N24	B 4N24 4N20	C - 2N20	LIGS 2 LEGS N12-200 ctrs 2 LEGS N12-300 ctrs

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## UNDERPINNING WITH SCREW PILE & BORED PIER Scale 1:20

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CENTRE OF SCREW PILE -MAX 400 OFF MASONRY WALL

 $\vdash$ 



## TYPICAL BORED PIER DETAIL Scale 1:20

DIAMETER - VERTICAL 'A' REINFORCEMENT - LIGATURES 'B'

> SECTION Ζ SCALE 1:20 -

> > 'B'

R10-300

R10-300

DESCRIPTION

HARD CLAY

#### BORED PIERS

		DIMENSIONS			CEMENT
			MIN. SOCKET		
MARK	DIAMETER	MIN. DEPTH	DEPTH	'A'	Έ
BP-A	450	1500	N/A	6-N20	R10
BP-B	450	6800	N/A	6-N20	R10

### **GROUND BEAMS LEGEND:**

DIMENSIONS			REINFORCEMENT				
MARK GB1 GB2	WIDTH 600 800	DEPTH 600 800	A 4N24 5N24	B 4N24 4N20	C - 2N20	LIGS 2 LEGS N12-200 ctrs 2 LEGS N12-300 ctrs	

NORTH POINT	SCALES AT A1		
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CLIENT			
ST. AM	BROSE AN	NGLICAN CHU	JRCH
ARCHITECT			
	JD,	A Co.	
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## INDICATIVE PROPPING SECTION Scale 1:20

PROPPING TO LAND ON GROUND WITH SUITABLE BEARING CAPACITY

### INDICATIVE MAIN ARCH PROPPING Scale 1:20

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